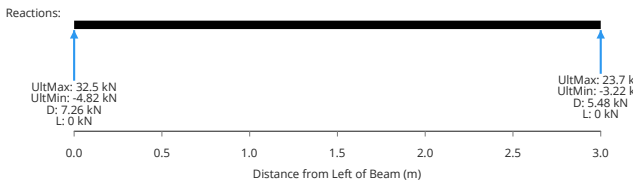
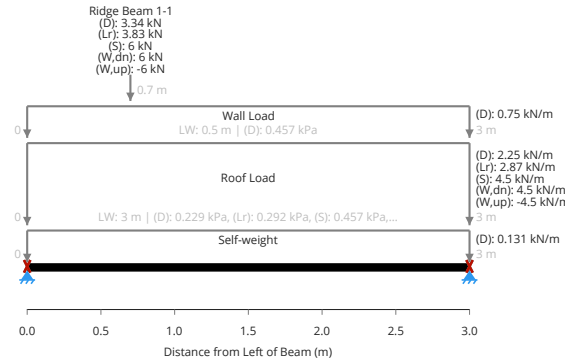




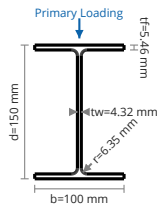
Client:	My Client	Date:	Oct 8, 2020
Author:	Brooks Smith	Job #:	12345
Project:	Example House Project	Subject:	Garage Lintel 1
References:	AISC 360-16 (LRFD)		

Summary

97%	Moment Capacity	$\phi M_n = 21.8 \text{ kN} \cdot \text{m}$
24%	Shear Capacity	$\phi V_n = 136 \text{ kN}$
61%	Critical Deflection	$\delta_{gov} = -5.09 \text{ mm}$
	Critical Deflection Ratio	$L/\delta = 589$
16%	Long-Term Deflection	$\delta_{LT} = -3.25 \text{ mm}$
	Critical Long-Term Deflection Ratio	$(L/\delta)_{LT} = 923$
33%	Simplified DL+LL Deflection	$\delta_{DL+LL} = -6.57 \text{ mm}$
	Critical Simplified DL+LL Deflection Ratio	$(L/\delta)_{DL+LL} = 457$
	Dead Load Deflection	$\delta_D = -3.32 \text{ mm}$



Key Properties



Designation	W6X9
Yield Strength	$F_y = 350 \text{ MPa}$
Total Length	$L = 3\,000 \text{ mm}$
Continuous Bracing for Lateral Torsional Buckling	No continuous bracing

Design Criteria

Design Code for Load Combinations	International Building Code (IBC) 2018
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Loads

Member Properties

Gross Area	$A_g = 1\,730 \text{ mm}^2$
Moment of Inertia	$I = 6\,830\,000 \text{ mm}^4$
Section Modulus	$S = 91\,100 \text{ mm}^3$
Elastic Global Buckling Length	$L_r = 2\,940 \text{ mm}$
Fully Plastic Length	$L_p = 967 \text{ mm}$

Section Classification (AISC 360-16 B4)

Section Classification - Flanges	Non-Compact
Section Classification - Web	Compact

Section Flexural Capacity (AISC 360-16, Chapter F)

Plastic Moment Resistance	$M_p = 35.7 \text{ kN} \cdot \text{m}$
Nominal Cross-section Bending Resistance	$M_{sxn} = 35.7 \text{ kN} \cdot \text{m}$

Flexural Capacity - Positive Bending (AISC 360-16, Chapter F)

Longest Unbraced Segment - Positive Bending	$L_{umax}^+ = 3\,000 \text{ mm}$
Lateral-Torsional Buckling Governs? - Positive Bending	$LTB_{flag}^+ = \text{Yes}$
Governing LTB Resistance - Positive Bending	$M_{LTB}^+ = 24.3 \text{ kN} \cdot \text{m}$

Flexural Capacity - Negative Bending (AISC 360-16, Chapter F)

Longest Unbraced Segment - Negative Bending	$L_{umax}^- = 3\,000 \text{ mm}$
Lateral-Torsional Buckling Governs? - Negative Bending	$LTB_{flag}^- = \text{Yes}$
Governing LTB Resistance - Negative Bending	$M_{LTB}^- = 24.3 \text{ kN} \cdot \text{m}$

Shear Capacity (AISC 360-16, Chapter G)

Shear Slenderness	$h/t_w = 29.2$
Web Shear Strength Coefficient	$C_{v1} = 1$
Nominal Shear Capacity	$V_n = 136 \text{ kN}$

Comments